GREENBRIER HALL ROOF RENOVATION

DRAWING INDEX

PROJECT TEAM

OWNER:

New River Community & Technical College

653 Church Street
Lewisburg, WV 24901

ARCHITECTURAL SERVICES:

Omni Associates, Architects

207 Jefferson Street
Fairmont, WV 26554

(304) 367-1417

Architect info
QR CODE

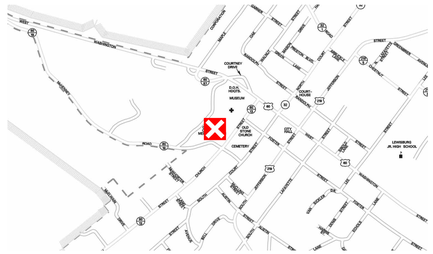
STRUCTURAL DESIGN:

Allegheny Design Services

102 Leeway Street
Morgantown, WV 26505

(304) 599-0771

PROJECT LOCATION



= **PROJECT LOCATION**

PROJECT PHASE GRAPHICS LEGEND

EXISTING | NEW | DEMOLISHED | TEMPORARY

GENERAL PROJECT NOTES

GENERAL NOTES APPLY TO ALL DRAWINGS

A. Do not scale the drawings.
B. Verify field conditions prior to commencement of each portion of the work. Some existing elements such as wall assemblies are unknown in composition, and thus, examination of the project existing conditions is required prior to bidding.
C. All dimensions are actual and are to face of studs, face of concrete walls, face of CMU walls, face of frames, or centerline of columns, unless noted otherwise.
D. The owner shall be responsible for providing the contractor with rough-in information necessary to accommodate the installation of owner furnished and installed items.
E. All dimensions to existing elements are approximate. confirm in field.

DRAWING SYMBOLS

EXTERIOR ELEVATIONS — BUILDING SECTIONS

INTERIOR ELEVATIONS — WALL & DETAIL SECTIONS

ROOM TAG — CEILING TAG

NORTH ARROW — STRUCTURAL GRIDS

PHOTO REFERENCE — CALLOUT REFERENCE

SIGNAGE TYPE — KEYED NOTES

ROOF or FLOOR ASSEMBLY TAG — GLAZING PANEL TYPE

DOOR TAG — TOILET ACCESSORIES

WALL ASSEMBLY TAG — MATERIAL TAG

WINDOW TAG — ELEVATION CHANGE

EQUIPMENT TAG

SPOT ELEVATION — BUILDING LEVEL

ARCHITECTURAL ABBREVIATIONS

AB	ANCHOR BOLT
ABV	ABOVE
ACT	ACOUSTICAL CEILING TILE
ADD	ADDITIONAL
ADJ	ADJACENT
AFF	ABOVE FINISH FLOOR
ALT	ALTERNATE
ALUM	ALUMINUM
APPROX	APPROXIMATE, APPROXIMATELY
ARCH	ARCHITECTURAL, ARCHITECT
AUTO	AUTOMATIC
BD	BOARD
BKHD	BULKHEAD
BL	BUILDING LINE, BASE LINE
BLDG	BUILDING
BOT	BOTTOM
BRG	BEARING
BTW	BETWEEN
CFMF	COLD-FORMED METAL FRAMING
CH	CHANNEL
CJ	CONTROL JOINT
CL	CENTER LINE
CLG	CEILING
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONT	CONTINUOUS
CTR	CENTER
DBL	DOUBLE
DEPT	DEPARTMENT
DIA	DIAMETER
DN	DOWN
DR	DOOR
DS	DOWNSPOUT
DTL	DETAIL
DWG	DRAWING
EA	EACH
EJ	EXPANSION JOINT
EL	ELEVATION
EQ	EQUAL
EQUIP	EQUIPMENT
EW	EACH WAY
EWC	ELECTRIC WATER COOLER
EXIS	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
FCB	FIBER-CEMENT BOARD
FD	FLOOR DRAIN
FE	FIRE EXTINGUISHER
FEC	FIRE EXTINGUISHER CABINET
FF	FINISHED FLOOR
FFE	FINISHED FLOOR ELEVATION
FG	FINISH GRADE
FIN	FINISH, FINAL
FLEX	FLEXIBLE
FLR	FLOOR
FOB	FACE OF BRICK
FOC	FACE OF CONCRETE
FOE	FACE OF EXISTING
FOS	FACE OF STUD
FR	FIRE RESISTANT
FRP	FIBER-REINFORCED PLASTIC
FRT	FIRE-RETARDANT TREATED
FTG	FOOTING
FV	FIELD VERIFY
GA	GAUGE
GALV	GALVANIZED
GC	GENERAL CONTRACTOR
GFI	GROUND FAULT INTERRUPTER
GWB	GYPSUM WALLBOARD
GYP	GYPSUM
HC	HANDICAP
HDW	HARDWARE
HM	HOLLOW METAL
HOR	HORIZONTAL
HP	HIGH PRESSURE
HR	HOUR
HSS	HIGH STRENGTH STEEL
HT	HEIGHT
HYD	HYDRANT
IAW	IN ACCORDANCE WITH
ID	INSIDE DIAMETER
INSUL	INSULATION
INT	INTERIOR
MANF	MANUFACTURER
MATL	MATERIAL
MAX	MAXIMUM
MECH	MECHANICAL
MH	MANHOLE
MIN	MINIMUM
MO	MASONRY OPENING
MR	MOISTURE RESISTANT
MTD	MOUNTED
MTG	MOUNTING
MTL	METAL
NIC	NOT IN CONTRACT
NTS	NOT TO SCALE
OC	ON CENTER
OD	OUTSIDE DIAMETER
OFCI	OWNER FURNISHED CONTRACTOR INSTALLED
OH	OPPOSITE HAND
OPG	OPENING
OPP	OPPOSITE
OVHD	OVERHEAD
P-LAM	PLASTIC LAMINATE
PEMB	PRE-ENGINEERED METAL BUILDING
PL	PLATE
PNL	PANEL
PR	PAIR
PT	PRESSURE TREATED
PWD	PLYWOOD
R	RISER
RAF	RAISED ACCESS FLOOR
RD	ROOF DRAIN
REF	REFERENCE
REINF	REINFORCING, REINFORCED
REQD	REQUIRED
REV	REVISION
RM	ROOM
RO	ROUGH OPENING
SAB	SOUND ATTENUATION BLANKET
SHT	SHEET
SIM	SIMILAR
SPEC	SPECIFICATIONS
SQ	SQUARE
SS	STAINLESS STEEL
STD	STANDARD
STL	STEEL
STOR	STORAGE
STRUC	STRUCTURE, STRUCTURAL
SURF	SURFACE
SW	SIDEWALK
T	TREAD
TEMP	TEMPERATURE, TEMPERED
TG	TONGUE & GROOVE
THD	THRESHOLD
TO	TOP OF
TOM	TOP OF MASONRY
TOS	TOP OF STEEL
TOW	TOP OF WALL
TRTD	TREATED
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
UTIL	UTILITY
VB	VAPOR BARRIER
VCT	VINYL COMPOSITION TILE
VER	VERTICAL
VTR	VENT THROUGH ROOF
VWC	VINYL WALL COVERING
WC	WATER CLOSET
WD	WOOD
WF	WIDE FLANGE
WH	WATER HEATER
WO	WITHOUT
WRB	WEATHER-RESISTIVE BARRIER
WT	WEIGHT
WWF	WELDED WIRE FABRIC
XP	EXPLOSION PROOF
YD	YARD DRAIN

STATE OF WEST VIRGINIA
RICHARD T. FORREN No 2230 REGISTERED ARCHITECT
2026-04-01

BID DOCUMENTS
2026-04-01
REVISIONS ▼

GREENBRIER HALL ROOF RENOVATION

NEW RIVER COMMUNITY & TECHNICAL COLLEGE
653 Church Street
Lewisburg, WV 24901

COVER SHEET

OMNI ARCHITECTS

G-1.0

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Project No: 2024008.002 **Drawn by:** ---

GENERAL STRUCTURAL NOTES:

1. Should a discrepancy occur between the construction drawings and the project technical specifications in the Project Manual, the specifications (including geotechnical report) will govern.

2. Design Code: IBC 2018, Risk Category II

3. No provisions have been made for future horizontal or vertical expansion.

4. Live Load data:

Uniform Floor Live Loads:

Location	Load
• Corridors	80 psf
Concentrated Live Loads (distributed over an area 2-1/2 sq. ft., unless noted otherwise):	
• Office Buildings:	2000 lbs
• Ladder:	500 lbs.

Handrail:

- Top Rail: 200 lb. or 50 lb/ft applied non-concurrently in any direction.

5. General Contractor shall verify all dimensions and conditions related to existing construction, existing services, and the site.

6. Construction loads shall not exceed design live loads. Shoring and re-shoring is the responsibility of the General Contractor.

SHOP DRAWINGS:

1. See Project Manual - Administrative Requirements for submittal procedures.

2. The General Contractor shall review, check, and stamp "Approved" all shop drawings prior to submitting them to the Architect. Shop drawings which have not been stamped "Approved" by the General Contractor do not conform to the requirements of the Contract Documents and will be rejected.

3. The General Contractor shall provide a shop drawing submittal schedule to the Architect at least two weeks prior to submittal of the first set of shop drawings.

4. Shop drawing submittals and review must precede the start of fabrication. General Contractor proceeds at their own risk without reviewed as noted shop drawings from the Architect. The maximum turn-around time for shop drawings will be two weeks (ten working days) from the date of receipt in the Architect's office to the date of return delivery. General Contractor and Architect shall increase coordination efforts when large numbers of shop drawings are invited.

5. Refer to the Project Manual (if available) for specific submittal procedures.

6. Shop drawings shall not simply be a copy of the engineer's design drawings with the contractor's title block. Electronic files from the E.O.R. can be provided to the G.C. at a negotiated fee. Fee will be based on amount of subcontractor's contract and benefit to subcontractor.

7. Shop drawings shall be completely prepared by the submitting entity.

STRUCTURAL STEEL WELDING

1. All welding shall be in strict conformance with latest building code and AWS D1.1

CAST-IN-PLACE CONCRETE:

1. Cast-in-place concrete work shall conform to the American Concrete Institute codes and standards listed in the Project Manual except as modified therein or on the drawings. Maintain copy of each on job site during construction.
2. The minimum ultimate compressive strength of concrete at 28 days shall be:

   a. 4000 psi - All other unless noted otherwise on drawings.

3. Air Content: All concrete exposed to freezing and thawing and/or required to be watertight shall have an air content of 4.5% to 7.5%. All other concrete shall have an air content of 3% to 4%.

4. Water Cement Ratio: All concrete subjected to exposed to freezing and thawing in moist condition and/or required to be watertight shall have a maximum water-cement ratio of 0.45. All reinforced concrete exposed to deicing salts, brackish water seawater or spray from these sources, shall have a maximum water-cement ratio of 0.40.

5. Maximum aggregate size shall be 1 1/2 ", well graded, well-shaped (not elongated, flat, or slippery), and free of clay, dirt, and excess fines, U.N.O.

6. Aggregate composition shall consist of quartz, limestone, dolomite, granite, or feldspar.

7. Cement shall be Type 1, U.N.O.

8. Maximum concrete slump 3", U.N.O.

9. Reinforcing bars: ASTM A615, Grade 60.

10. Welded wire fabric: ASTM A185.

11. Provide 6x6-w2.9xw2.9 welded wire fabric in all non-structural slabs on grade, unless otherwise noted.

12. Place reinforcement in slabs, 1-1/2" down from top of slab, unless otherwise noted.

13. Provide control joints in all non-structural slabs on grade. The maximum spacing of control joints shall be 20'-0" O.C. unless otherwise noted. Control joint depth equal to 1/5 slab thickness not less than 1 inch.

14. Reinforcing bar lap splices and anchorage lengths shall conform with ACI 318-11 "Development and Splices of Reinforcement." All splices shall be Type B.

15. Top layer of reinforcing steel in slabs and footings shall be considered top bars regardless of thickness of concrete below the bars.

16. All horizontal wall bars shall be bent lapped around all corners, unless otherwise noted.

17. Provide vertical and horizontal reinforcing bars in concrete walls to conform to the minimum provisions of ACI 318, Section 14.3 unless otherwise noted.

18. Chamfer exposed edges of concrete 1/2" unless otherwise noted.

19. Refer to architectural drawings for location and extent of finishes or other treatments to exposed concrete.

20. Determine size, location and weight of mechanical equipment and make provisions for bolts, sleeves, pads, etc. from manufacturer's certified drawings. This work shall be coordinated with the trades involved.

21. All new concrete shall be bonded to previously placed concrete per specification requirements, U.N.O.

22. The Contractor shall prepare shop drawings showing detail layouts of reinforcing, including dimensions, openings, and spacing, bending details, bar schedules, and similar items required for the proper construction of the work.Provisions for the connection of work by other trades shall be indicated on the shop drawings. The location of all embedded items shall be indicated by the contractor on the shop drawings. All shop drawings shall be submitted for approval in accordance with the requirements of the Contract Documents.

23. Preparing, curing, transporting, and testing concrete cylinders. For each class of concrete placed, at least four cylinders shall be taken for each 50 cubic yards, or fraction thereof, of each class of concrete placed each day. Cylinders are to be taken in accordance with ASTM C31 and results shall be submitted to the Architect/Engineer, Construction Manager and owner. Two cylinders will be tested at 7 days and two at 28 days.

STRUCTURAL STEEL:

1. All structural steel work shall be in accordance with the "Specifications for the Design, Fabrication, and Erection of Structural Steel Buildings" (14th Edition) of the AISC. Maintain copy of each on job site during construction.

2. Structural steel shall conform to the following:

   a. Wide flange shapes and WT's - ASTM A992 with a minimum yield strength of 50,000 PSI.

   b. Channels, angles, plates, and miscellaneous connection material - ASTM A36 with a minimum yield strength of 36,000 PSI unless noted otherwise.

   c. Pipes - ASTM A501 with a minimum yield strength of 36,000 PSI or ASTM A53 Type E or S with a minimum yield strength of 35,000 PSI.

   d. Tubes - ASTM A500, Grade B with a minimum yield strength of 46,000 PSI.

3. All bolts shall be 3/4" dia. unless noted otherwise ASTM A325 H.S. bolt of either friction or bearing type. Use slip critical connections for all wind bracing connections. Threads shall be included in the shear plane.

4. All bolted connections shall be made according to AISC Table II or III framed beam connections. The minimum depth of connection must be more than one half the depth of the beam except that beams framing to columns shall have full depth connections using 3/8" connection angles or plates. Contractor shall provide certified design for all shear connections by a Professional Engineer in the state in which the project is located. Submit calculations for moment connections using braced member capacity U.N.O. on plans. Minimum end reaction of beams:

   - W8's = 10 kips
   - W10's= 12 kips
   - W12's= 16 kips
   - W14's= 18 kips
   - W16's= 20 kips
   - W18's= 22 kips
   - W21's= 24 kips
   - W24's= 26 kips

5. All welding shall be in strict accordance with the standards of the AWS and the AISC. Use E70XX electrodes.

6. Contractor shall submit Welders' certificates in accordance with AWS B2.1/B2.1M. It shall be dated no more than 12 months before start of scheduled welding work.

7. Do not paint steel where encased in concrete or at field weld areas.

8. No shop or field holes or cuts are to be placed in structural members unless indicated on the contract or shop drawings.

9. The Structural Steel Fabricator shall field verify all dimensions prior to fabrication. Particularly for stairs, handrail systems, etc.

10. The Structural Steel Fabricator shall provide for vertical and horizontal adjustment of all support assemblies.

11. The Structural Steel Fabricator and/or the General Contractor shall verify all existing dimensions and conditions at the site. All discrepancies found shall be reported to the Architect prior to preparation of shop drawings. Shop drawings shall include all field measurements and conditions.

12. Expansion bolts:Use expansive anchors of the diameter indicated on the drawings as manufactured by HILTI Fastening Systems or approved equal.

    a. In concrete, use Kwik Bolt TZ2 .

    b. In brick and CMU, use sleeve and fill CMU cells at all bolt locations.

13. Anchor bolts must meet ASTM A1554 gr. 36 specifications and be 3/4" diameter (unless otherwise indicated) .

14. The Structural Steel Fabricator shall provide all supports for any precast concrete panels as indicated on the drawings and as required by the manufacturer. Location of such supports shall be coordinated with the manufacturer.

15. The Structural Steel Fabricator, Precast Concrete Supplier, and the General Contractor shall coordinate all support and tie-back locations and connections, if applicable.

16. All galvanizing shall be per ASTM A123 and A780. All steel exposed to the elements and masonry support members shall be galvanized. Backup steel supporting masonry veneer and precast support angles shall be zinc primed and painted.

17. Refer to architectural and mechanical drawings for possible miscellaneous steel. This steel shall also conform to the requirements in these General Notes and the Structural Steel specifications.

18. 75% of field welds shall be visually inspected and tested using one of the following:
    A. Radiographic testing performed in accordance with ASTM E94/E94M
    B. Ultrasonic testing performed in accordance with ASTM E164
    C. Liquid penetrant inspection performed in accordance with ASTM E165/165M
    D. Magnetic particle inspection performed in accordance with ASTM E709

19. Steel fabricator shall review Architectural drawings and include all miscellaneous steel in their bid. If notes on architectural drawings refer to "see structural" and the structural drawings do not address this item notify the E.O.R. at least two weeks prior to bid opening to allow time for issue of addendum.

20. Column Schedule may not include all columns on the project. Review all drawings to insure all columns are included in bid.

21. Steel lintels can be replaced with engineered reinforced precast lintels. Mfgr shall submit certified Engineer design with lintel submittal.

22. Support for RTU's shall be limited to framed opening steel around roof openings. Additional steel required for RTU support shall be designed and provided by the General Contractor with coordination with the RTU manufacturer.

PERFORMANCE REQUIREMENTS:

1. No provision of any referenced standard specification, manual or code (whether or not specifically incorporated by reference in the contract documents) shall be effective to change the duties and responsibilities of owner, contractor, engineer, supplier, or any of their consultants, agents, or employees from those set forth in the contract documents. Nor shall it be effective to assign to the structural engineer of record or any of the structural engineer of record's consultants, agents, or employees ant duty or authority to supervise or direct the furnishing or performance of the work or any duty or authority to undertake responsibilities contrary to the provisions or the contract documents.

2. Contract documents include, but are not limited to, the structural documents (drawings and specifications), but do not include shop drawing, vendor drawings, or materials prepared and submitted by the contractor.

3. Reference to standard specifications or any technical society, organization, or association or to codes of local or state authorities, shall mean the latest standard, code, specification or tentative specification adopted at the date of taking bids, unless specifically stated otherwise.

4. Contract documents shall govern in the event of a conflict with the code of practice or specifications of ACI, PCI, AISC, SJI, or other standards. Where a conflict occurs within the contract documents, the strictest requirement shall govern.

5. Contractor shall obtain and coordinate edge of slab and roof deck edge dimensions, opening locations and dimensions, depressed slab locations and extents, slab slopes, curb locations, and CMU wall location. Architect/Structural engineer shall be notified of any discrepancy or omission. In the event of discrepancies, the non-structural architectural details shall govern.

6. Contractor shall verify existing dimensions, elevations, and site conditions before starting work. Architect/Structural engineer shall be notified of any discrepancy.

7. Contractor shall verify the structurally supported mechanical equipment weights, opening sizes, and locations identified on the structural drawing with architectural and mechanical drawings.

8. Contractor shall verify that miscellaneous framing shown on the structural drawings for mechanical equipment, owner-furnished items, partitions, etc. is consistent with the requirements of such items.

9. Contractor has responsibility for means, methods, safety, techniques, sequences, and procedures of construction.

10. The structure is stable only in its completed form. Temporary supports required for stability during all intermediate stages of construction shall be designed, furnished, and installed by the contractor. Contractor is responsible for constructability analysis, and erection procedures, including design and erection of framework, temporary bracing, etc.

11. Contractor has sole responsibility to comply with all OSHA regulations.

12. Reproduction of structural drawings for shop drawings is not permitted.

13. Electronic drawing files will not be provided to the contractor.

14. Structural engineer of record is not responsible for the design of steel stairs, handrails, curtain wall/window systems, cold-formed metal framing, toilet partition supports, shelf systems, or other systems not shown in the structural documents, such systems shall be designed, furnished, and installed by others as required by other portions of the contract documents.

15. Structural Engineer shall not be responsible for specifying all waterproofing details and elements on the superstructure and below grade structures.

16. General Contractor shall review and coordinate elevator rail and hoist requirements with structural drawings. Notify structural engineer immediately if changes are required.

DESIGN/CONSTRUCTION DOCUMENTS - EXISTING CONSTRUCTION:

Design and construction documents are based upon assumptions made from available original and field investigation. Information shown may not necessarily reflect actual conditions. The contractor drawings shall field verify or establish the following:

1. All dimensions and elevations.

2. Existing conditions.

3. Existing structural arrangement and sizes in work areas.

The contractor shall notify the Architect of any discrepancies between the design documents and actual conditions. All existing dimensions and conditions shall be reflected on the shop drawings prior to submission for review.

STRUCTURAL SYMBOLS AND HATCHING	
SYMBOL	EXPLANATION
	MOMENT CONNECTION
	CANTILEVER MOMENT CONNECTION
	DIRECTION OF DECK SPAN
	BEAM BEARING PLATE
	TEFLON BEAM BEARING PLATE
	SPOT ELEVATIONS
	EXPANSION JOINT
	CONCRETE WALL
	MASONRY WALL
	AREA TO RECEIVE PAVERS

DRAWING LIST	
DRAWING NUMBER	DRAWING NAME
S001	GENERAL NOTES
S100	PLANS

JASON D. ROBINSON
REGISTERED
19755
4-1-2026
STATE OF WEST VIRGINIA
PROFESSIONAL ENGINEER

BID DOCUMENTS
2026-04-01
REVISIONS ▼

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GREENBRIER HALL ROOF RENOVATION

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GENERAL NOTES

OMNI ARCHITECTS

S001

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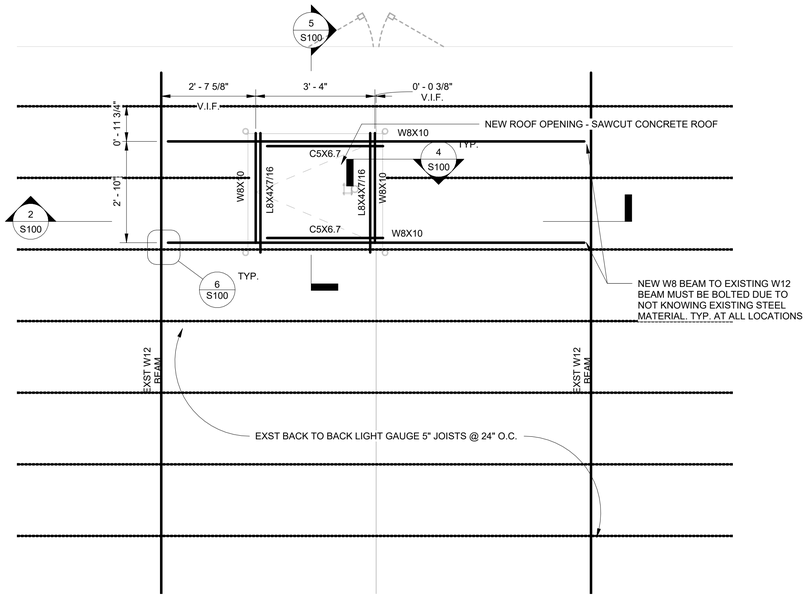
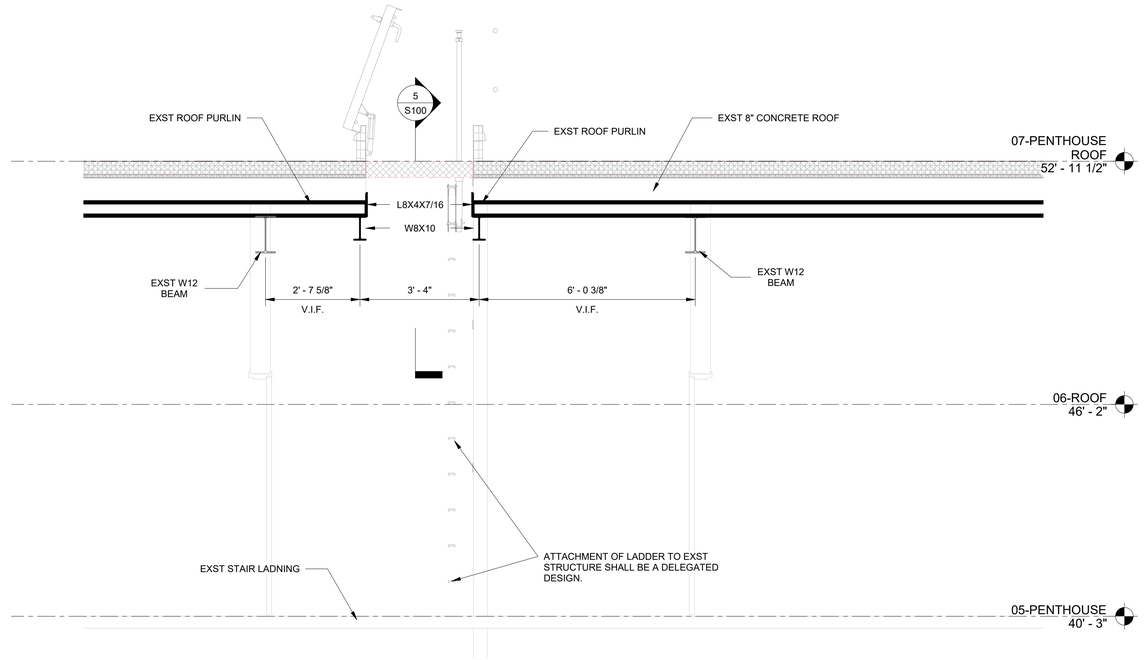


1 S100 **PENTHOUSE ROOF OPENING PLAN**

SCALE: 1/2" = 1'-0"

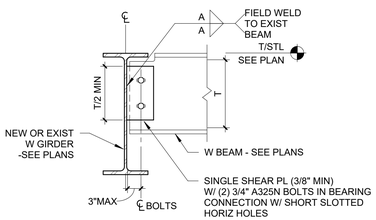
NOTES:

1. ELEVATIONS REFERENCE FINISH FLOOR. REFERENCE LEVEL = 0'-0" (LEVEL 1)
2. REFER TO ARCHITECTURAL DRAWINGS FOR ROOF HATCH LOCATION.
3. EXISTING WIDE FLANGE MEMBER SIZES ARE UNKNOWN, NOTIFY EOR IF CONNECTIONS WILL NOT FIT. ALL CONNECTIONS TO EXISTING STEEL MUST BE BOLTED.
4. EXISTING STEEL GRADE IS UNKNOWN. ALL CONNECTIONS TO EXISTING STEEL MUST BE BOLTED.
5. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL EXISTING CONDITIONS, DIMENSIONS, ETC. BEFORE CONSTRUCTION. IF DISCREPANCIES ARISE, NOTIFY EOR IMMEDIATELY.
6. ANY DAMAGE TO THE EXISTING FINISH WILL REQUIRE THE FINISH TO BE REPAIRED - SEE ARCHITECTURAL.

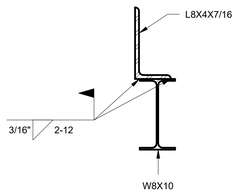


2 S100 **ROOF HATCH SECTION**

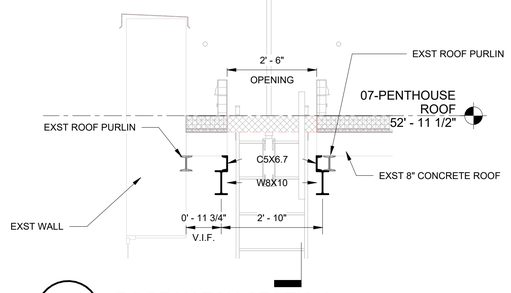
SCALE: 1/2" = 1'-0"



3 S100 **BEAM CONNECTION TO BEAM**

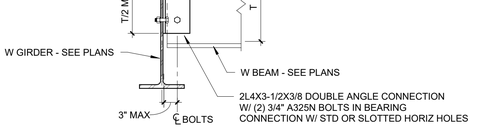


4 S100 **ANGLE TO WIDE FLANGE CONNECTION DETAIL**



5 S100 **ROOF HATCH SECTION**

SCALE: 1/2" = 1'-0"



6 S100 **TYPICAL FRAMED BEAM CONNECTION**

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4-1-2026
STATE OF WEST VIRGINIA
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PLANS

OMNI ARCHITECTS

S100

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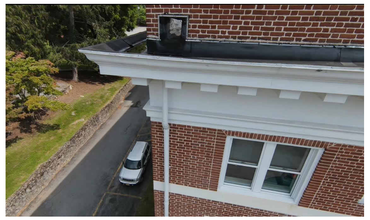


01 SCUPPER 1 - NW

02 SCUPPER 2 - N

03 SCUPPER 3 - N

04 SCUPPER 4 - NE

05 SCUPPER 5 - SE

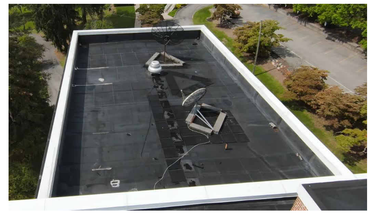
06 SCUPPER 6 - S

07 SCUPPER 7 - S

08 SCUPPER 8 - SW

09 SCUPPER 1 (NW) INTERIOR

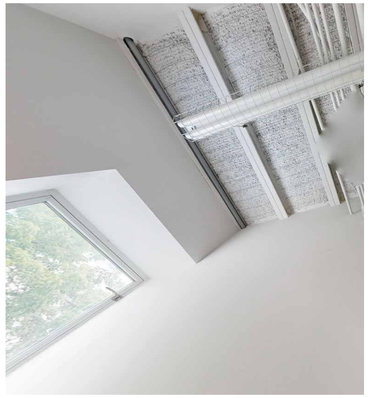
10 PARAPET WALL AND CAP



11 WEST ROOF - EQUIPMENT
SCALE: 12" = 1'-0" SHEET: AE-0.1

12 EAST ROOF - EQUIPMENT
SCALE: 12" = 1'-0" SHEET: AE-0.1

13 PENTHOUSE ROOF - EQUIPMENT
SCALE: 12" = 1'-0" SHEET: AE-0.1



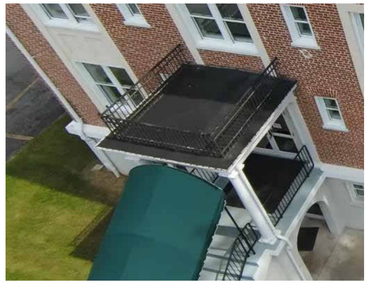
14 LADDER/HATCH LOCATION

15 PENTHOUSE ROOF - PARAPET

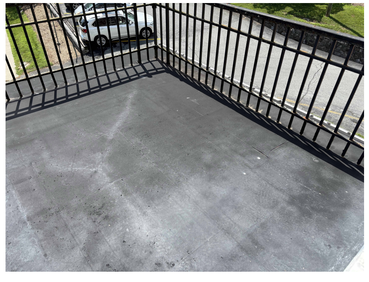
16 SIDE ROOF - EAST

17 SIDE ROOF - EAST - RAILING

18 SIDE ROOF - EAST - SOFFIT

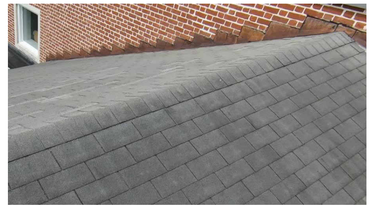


19 SIDE ROOF - WEST
SCALE: 12" = 1'-0" SHEET: AE-0.1



20 SIDE ROOF - WEST - RAILING
SCALE: 12" = 1'-0" SHEET: AE-0.1

21 SIDE ROOF - WEST - SOFFIT



22 GABLE ROOF - STEP FLASHING
SCALE: 12" = 1'-0" SHEET: AE-0.1

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GREENBRIER HALL ROOF RENOVATION

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EXISTING CONDITIONS

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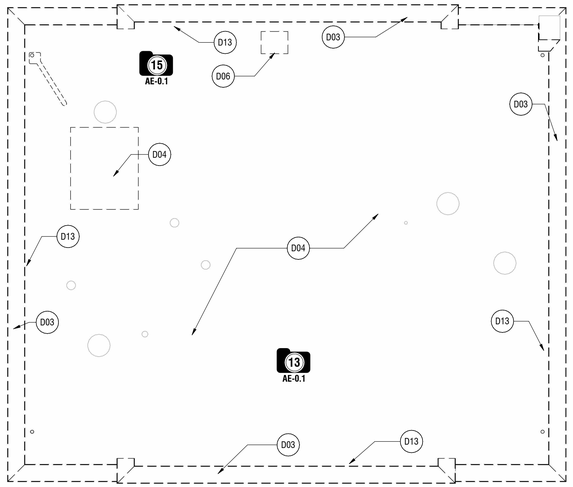
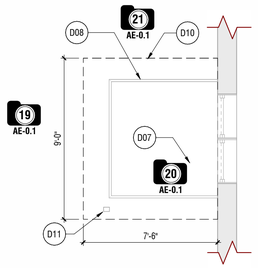
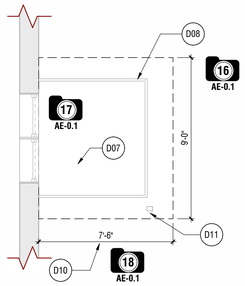


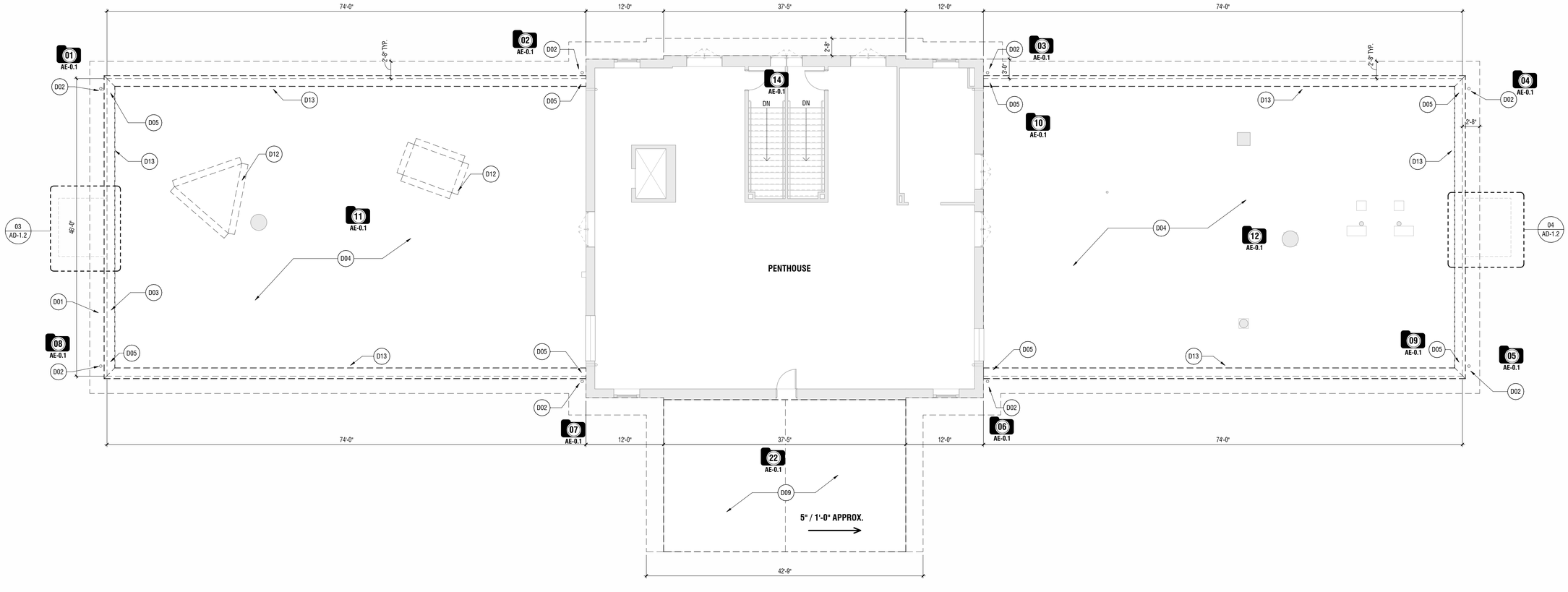
02 PENTHOUSE ROOF - DEMOLITION
SCALE: 1/8" = 1'-0" SHEET: AD-1.2



03 SIDE ROOF - WEST - DEMOLITION
SCALE: 1/4" = 1'-0" SHEET: AD-1.2



04 SIDE ROOF - EAST - DEMOLITION
SCALE: 1/4" = 1'-0" SHEET: AD-1.2



01 ROOF PLAN - DEMOLITION
SCALE: 1/8" = 1'-0" SHEET: AD-1.2

KEYED DEMOLITION NOTES

#	Note Text
D01	REMOVE EXISTING MEMBRANE AND TERMINATIONS FROM CORNICE IN THEIR ENTIRETY.
D02	REMOVE PORTIONS OF EXISTING DOWNSPOUT WHICH CONNECT TO CORNICE GUTTER WHERE DAMAGED/LEAKING AS NEEDED
D03	REMOVE EXISTING METAL COPINGS IN THEIR ENTIRETY.
D04	REMOVE EXISTING ROOF ASSEMBLY, LEAVING CONCRETE SLAB INTACT IN PREPARATION FOR NEW ROOFING MATERIAL. COORDINATE WITH NEW WORK DRAWINGS.
D05	REMOVE EXISTING MEMBRANE FROM EXISTING THRU-WALL SCUPPERS
D06	REMOVE PORTION OF EXISTING ROOF AND DECK IN PREPARATION FOR ROOF LADDER ACCESS HATCH, COORDINATE WITH NEW WORK DRAWINGS.
D07	REMOVE EXISTING BUILT-UP ROOFING AND SHEATHING IN ITS ENTIRETY IN PREPARATION FOR NEW MEMBRANE ROOF, COORDINATE WITH NEW WORK DRAWINGS.
D08	REMOVE EXISTING METAL RAILING IN ITS ENTIRETY. RESERVE FOR REINSTALLMENT.
D09	REMOVE EXISTING ASPHALT SHINGLE ROOF ASSEMBLY, LEAVING CONCRETE ROOF ASSEMBLY INTACT IN PREPARATION FOR NEW ROOFING MATERIAL AND EXISTING STEP FLASHING. COORDINATE WITH NEW WORK DRAWINGS.
D10	REMOVE DAMAGED PORTIONS OF EXISTING SOFFIT IN PREPARATION FOR REPLACEMENT WITH LIKE MATERIAL. COORDINATE WITH NEW WORK DRAWINGS.
D11	REMOVE PORTIONS OF EXISTING DOWNSPOUT WHICH CONNECT TO SIDE ROOF WHERE DAMAGED/LEAKING AS NEEDED
D12	REMOVE EXISTING SATELLITES AND CONCRETE BLOCKS
D13	REMOVE EXISTING SHEATHING ON BACK OF PARAPETS WHERE PRESENT. REFER TO SPECIFICATIONS FOR ALLOWANCES.

GENERAL DEMOLITION NOTES

A. Coordinate all demolition/phasing efforts with the architect, engineer and owner's representatives. Every effort shall be made to minimize disruption of adjacent operations and to ensure public safety and safe working conditions. Excessive noise or vibration shall be pre-approved and coordinated with the owner's representative.
B. Coordinate with the owner any pre-approved disruption and verification of service within the existing building so as to minimize the disruption of service.
C. Provide temporary dust partition protecting access enclosures where noted and as directed by the owner or architect around all areas of work to prevent the spread of construction debris and dust. Temporary partitions to be constructed prior to start of the demolition work. Isolate any demolition/construction work from the general public and maintain means of egress throughout the work.
D. Maintain a secure, weather-tight enclosure at the exterior of the existing building throughout the demolition/construction portion of the work in that area. Building security shall be coordinated with the owner.
E. Verify all existing conditions, dimensions, and elevations and notify the architect-engineer of any discrepancies.
F. Remove all demolition materials from the site unless noted otherwise. The owner shall reserve the right to salvage any materials.
G. Provide protection for all existing building materials and equipment from damage due to any demolition or construction-related incident performed under this contract.
H. Repair or replace any walls, floors, or equipment damaged as a result of demolition or construction to match existing finish and/or condition.
I. The contractor shall coordinate with the architect-engineer any materials to be reused and will be responsible for verifying and maintaining the functional and aesthetic integrity of the materials.
J. Verify and maintain the location of existing power, communication and data cables so as not to inadvertently interrupt the continuity of their service.
K. Patch all floor and ceiling penetrations resulting from removal or rerouting of new or existing piping, ductwork, conduit, etc...as required to maintain fire separations. Finish as required for new or existing adjacent surfaces.
L. Contractor to remove all unnecessary or abandoned piping, conduit and wiring that is accessible without additional demolition.
M. Any demolition work necessary on floor above or below (demolition required to install new work) shall be scheduled with the owner. Any finishes disturbed or damaged as a result of demolition or installation shall be patched or replaced to match existing finishes.

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ROOF DEMOLITION PLAN

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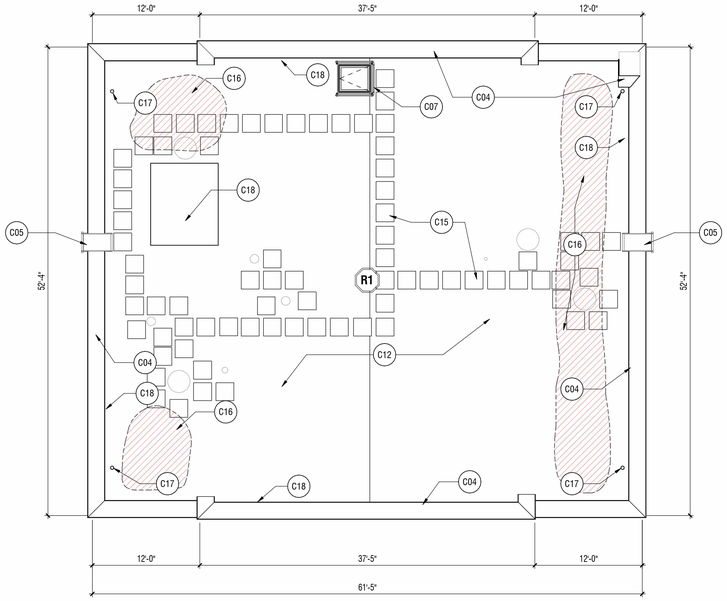
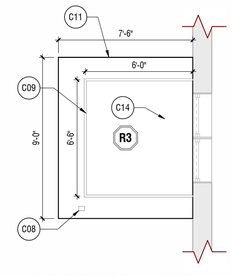
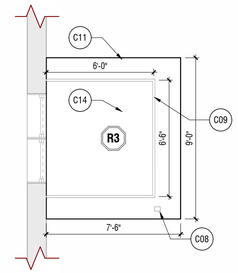


02 PENTHOUSE ROOF
SCALE: 1/8" = 1'-0" SHEET: A-1.2



03 SIDE ROOF - WEST
SCALE: 1/4" = 1'-0" SHEET: A-1.2



04 SIDE ROOF - EAST
SCALE: 1/4" = 1'-0" SHEET: A-1.2

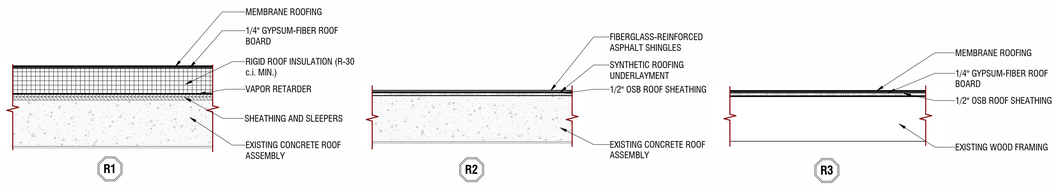
KEYED CONSTRUCTION NOTES

#	Note Text
C01	REPLACE ROOFING MEMBRANE AND TERMINATION STRIPS IN CORNICE GUTTER
C02	REPLACE PORTIONS OF DOWNSPOUT WHICH CONNECT TO CORNICE GUTTER WHERE DAMAGED/LEAKING AS NEEDED
C03	THRU-WALL SCUPPERS, SEE DETAILS
C04	INSTALL NEW METAL COPING OVER EXISTING STONE PARAPET CAP
C05	EXTERIOR 20" WIDE ALUMINUM WALL LADDER WITH WALK-THRU PLATFORM AND PARAPET RETURN
C06	20" WIDE ALUMINUM LADDER WITH SAFETY POST. LADDER STARTS AT LANDING AND EXTENDS TO ROOF HATCH.
C07	ROOF ACCESS HATCH WITH ATTACHED SAFETY RAILING
C08	REPLACE PORTIONS OF DOWNSPOUT WHICH CONNECT TO SIDE ROOF WHERE DAMAGED/LEAKING AS NEEDED
C09	REINSTALL ORIGINAL METAL RAILINGS.
C10	USE SELF-ADHERING ICE GUARD MEMBRANE AT ALL EDGES.
C11	REPLACE DAMAGED SOFFIT WITH LIKE MATERIAL AND PAINT TO MATCH EXISTING.
C12	REPLACE ROOFING MATERIAL ABOVE CONCRETE SLAB PER ROOF TYPE (SEE ASSEMBLIES).
C13	REPAIR EXISTING STEP FLASHING AS NEEDED AFTER REPLACING ROOFING MATERIAL.
C14	REPLACE ROOFING MATERIAL AND MEMBRANE PER ROOF TYPE (SEE ASSEMBLIES).
C15	NEW ROOF WALKWAY PADS
C16	THIS AREA OF ROOF HAS LOW SPOT. ADJUST SUBSTRATE AND NEW TAPERED INSULATION TO REPAIR
C17	4" DIAM. ROOF DRAINS AT CORNERS, TIE IN NEW MEMBRANE
C18	REPLACE SHEATHING ON BACK OF PARAPETS WHERE PRESENT. REFER TO SPECIFICATIONS FOR ALLOWANCES.

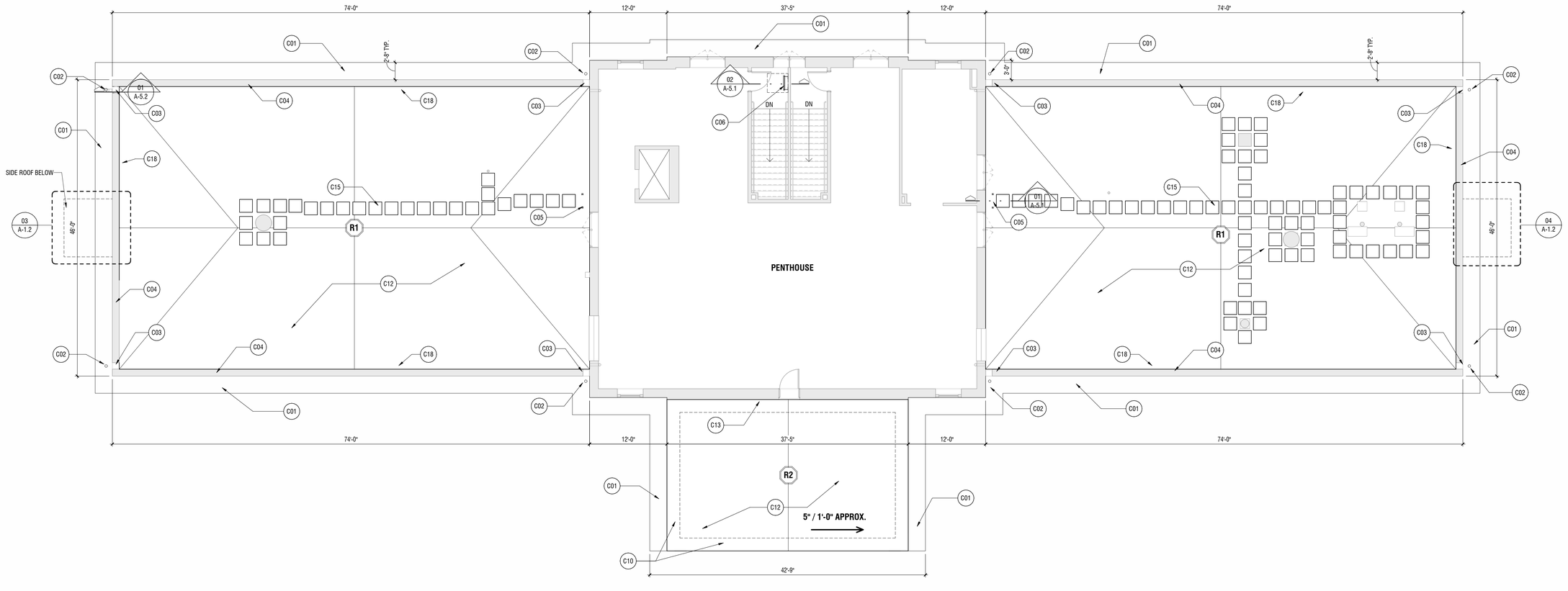
GENERAL PROJECT NOTES

GENERAL NOTES APPLY TO ALL DRAWINGS

A. Do not scale the drawings.
B. Verify field conditions prior to commencement of each portion of the work. Some existing elements such as wall assemblies are unknown in composition, and thus, examination of the project existing conditions is required prior to bidding.
C. All dimensions are actual and are to face of studs, face of concrete walls, face of CMU walls, face of frames, or centerline of columns, unless noted otherwise.
D. The owner shall be responsible for providing the contractor with rough-in information necessary to accommodate the installation of owner furnished and installed items.
E. All dimensions to existing elements are approximate. confirm in field.



05 ROOF ASSEMBLIES
SCALE: 1" = 1'-0" SHEET: A-1.2



01 ROOF PLAN
SCALE: 1/8" = 1'-0" SHEET: A-1.2

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ROOF PLAN

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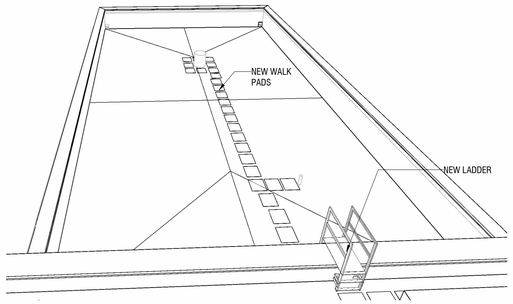
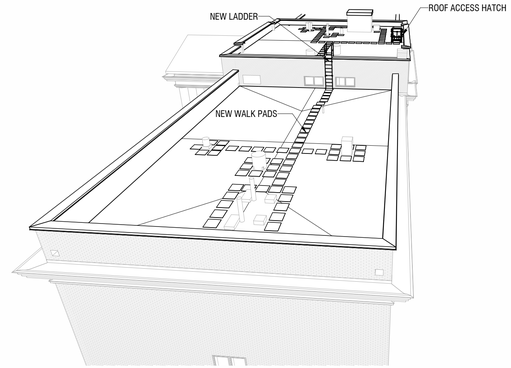
01 WEST ROOF



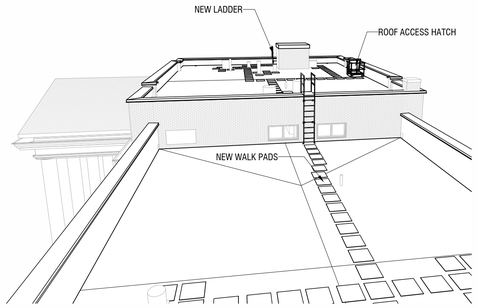
01a WEST ROOF

04 EAST ROOF

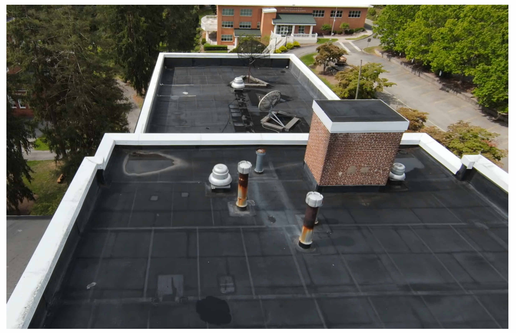


04a EAST ROOF

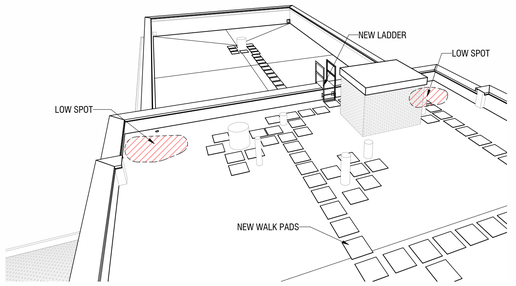
02 PENTHOUSE ROOF TRANSITION



02a PENTHOUSE ROOF TRANSITION

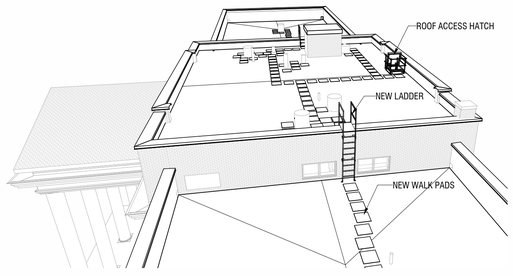


05 PENTHOUSE ROOF LOW SPOTS

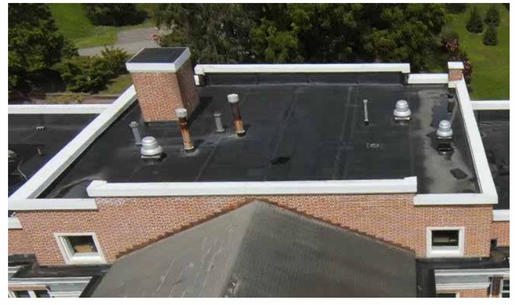


05a PENTHOUSE ROOF LOW SPOTS

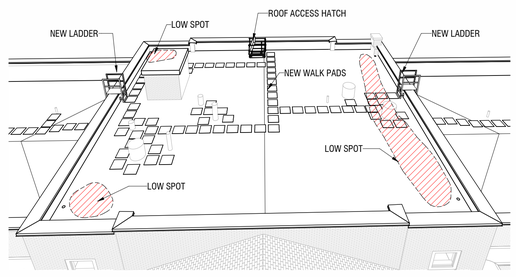
03 PENTHOUSE ROOF



03a PENTHOUSE ROOF



06 PENTHOUSE ROOF FRONT



06a PENTHOUSE ROOF FRONT

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3D VIEWS

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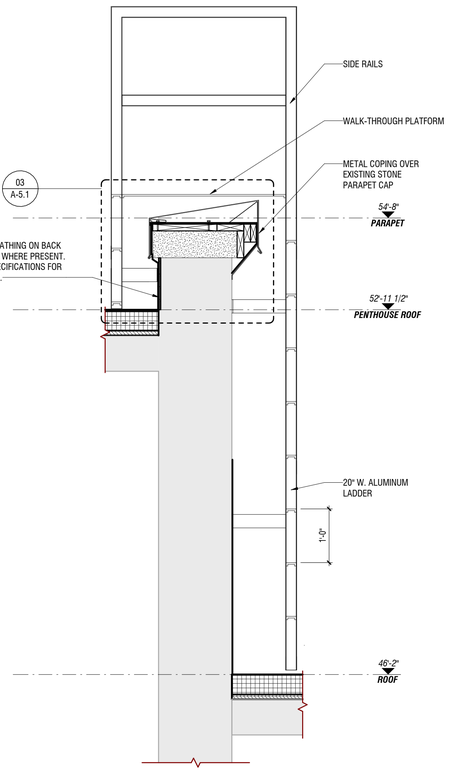
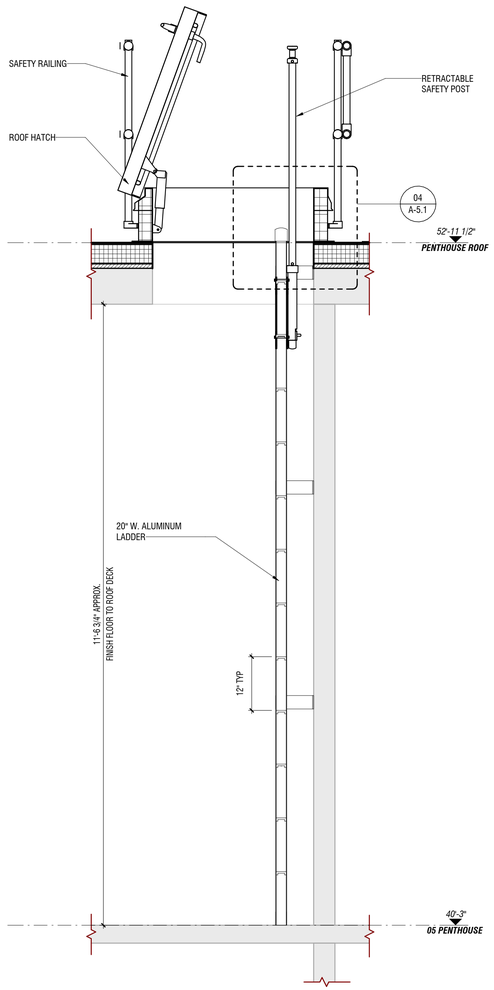


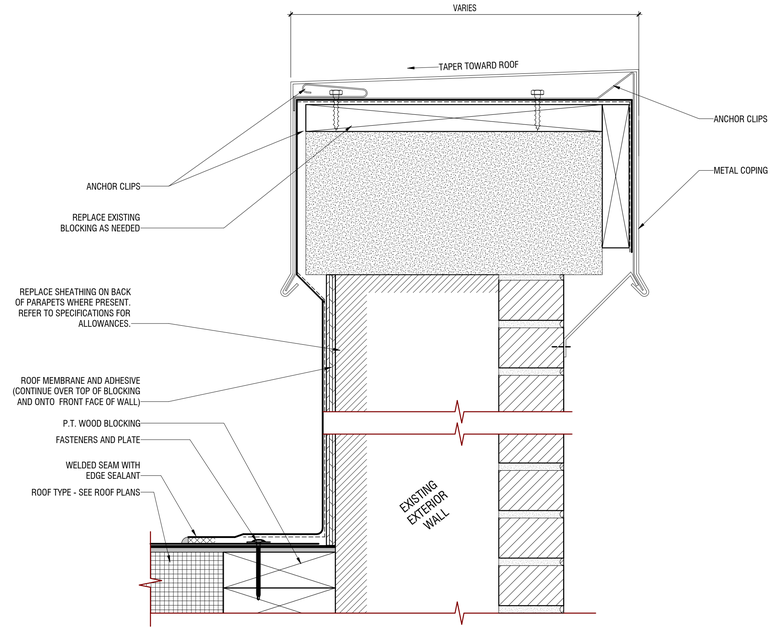
01 WALL LADDER AT PARAPET RETURN

SCALE: 3/4" = 1'-0" SHEET: A-5.1



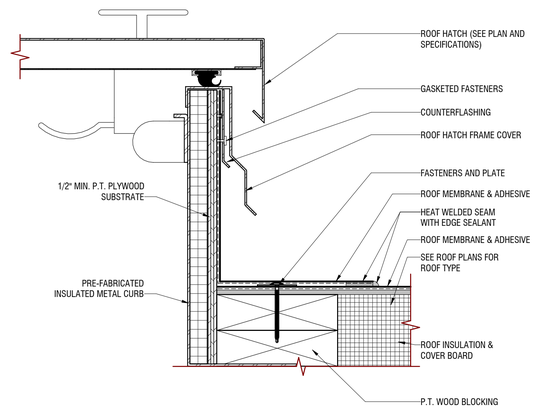
02 ROOF LADDER TO ROOF HATCH

SCALE: 3/4" = 1'-0" SHEET: A-5.1



03 TYP. PARAPET COPING AND BASE TERMINATION

SCALE: 3" = 1'-0" SHEET: A-5.1



04 TYP. ROOF HATCH CURB DETAIL

SCALE: 3" = 1'-0" SHEET: A-5.1

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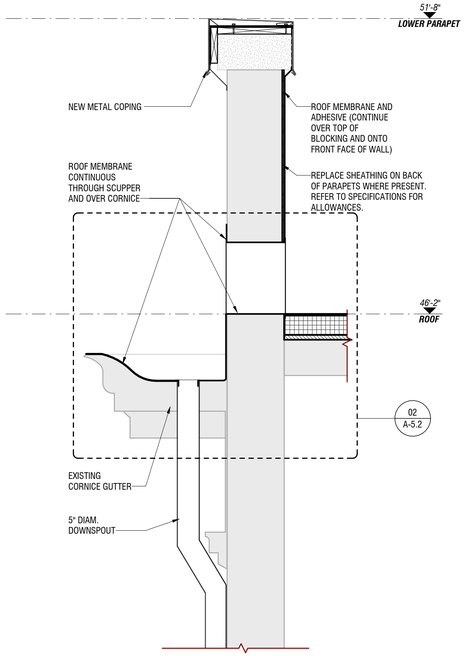
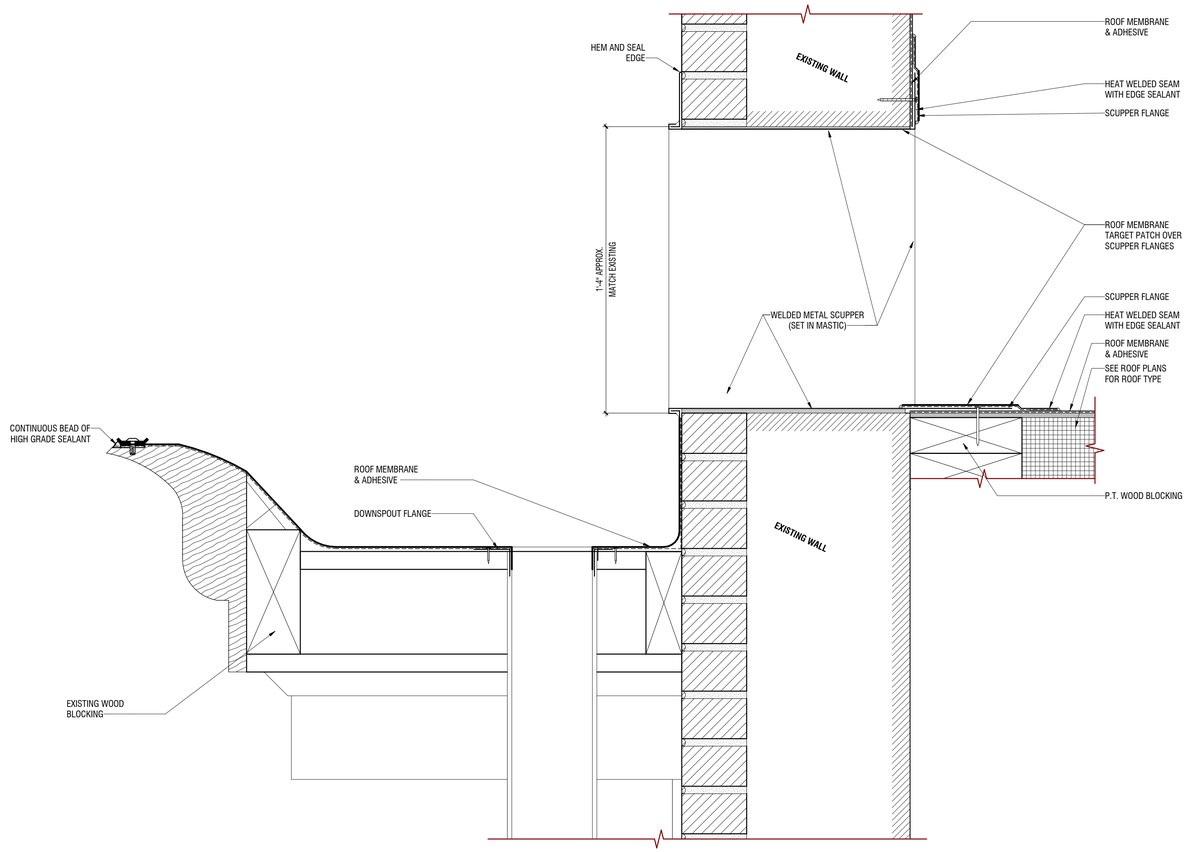


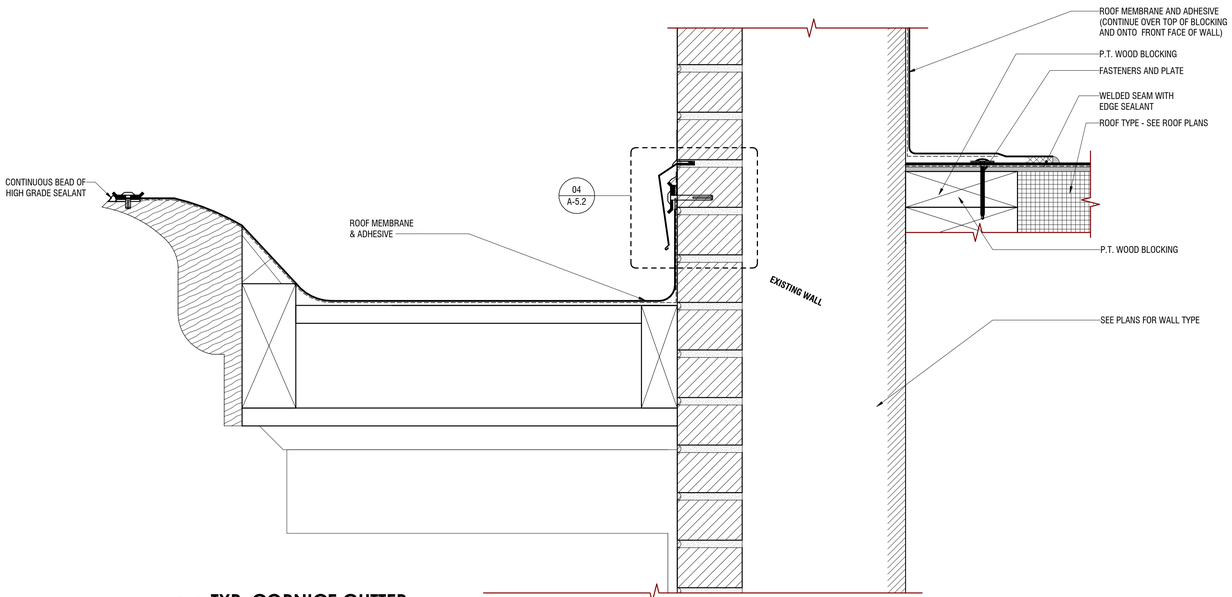
01 TYP. CORNICE GUTTER AND DOWNSPOUT

SCALE: 3/4" = 1'-0" SHEET: A-5.2



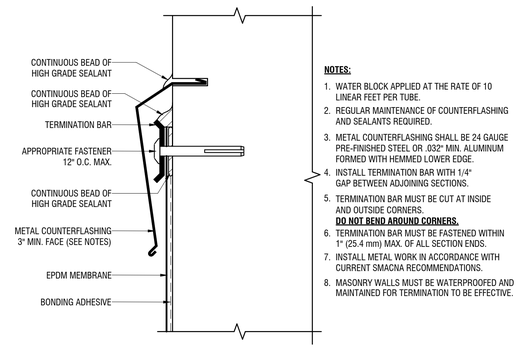
02 TYP. THRU-WALL SCUPPER AT CORNICE GUTTER

SCALE: 3" = 1'-0" SHEET: A-5.2



03 TYP. CORNICE GUTTER

SCALE: 3" = 1'-0" SHEET: A-5.2



NOTES:

1. WATER BLOCK APPLIED AT THE RATE OF 10 LINEAR FEET PER TUBE.
2. REGULAR MAINTENANCE OF COUNTERFLASHING AND SEALANTS REQUIRED.
3. METAL COUNTERFLASHING SHALL BE 24 GAUGE PRE-FINISHED STEEL OR .032" MIN. ALUMINUM FORMED WITH HEMMED LOWER EDGE.
4. INSTALL TERMINATION BAR WITH 1/4" GAP BETWEEN ADJOINING SECTIONS.
5. TERMINATION BAR MUST BE CUT AT INSIDE AND OUTSIDE CORNERS. **DO NOT BEND AROUND CORNERS.**
6. TERMINATION BAR MUST BE FASTENED WITHIN 1" (25.4 mm) MAX. OF ALL SECTION ENDS.
7. INSTALL METAL WORK IN ACCORDANCE WITH CURRENT SMACNA RECOMMENDATIONS.
8. MASONRY WALLS MUST BE WATERPROOFED AND MAINTAINED FOR TERMINATION TO BE EFFECTIVE.

04 TERMINATION BAR AND REGLET

SCALE: 6" = 1'-0" SHEET: A-5.2

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